

Modeling Extreme Events from Computer Simulations Part II

Emmanuel Vazquez

SUPELEC, Gif-sur-Yvette, France

Summer School CEA-EDF-INRIA, 2011

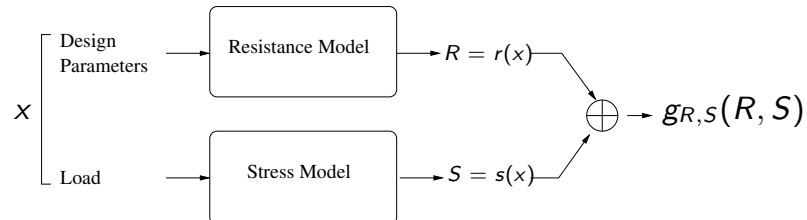
Outline of Part II

- 1** Structural reliability
 - Limit-state functions
 - Reliability indexes
 - Limit-state function defined on the factor space
 - Affine limit-state functions
 - First-order reliability method
 - Reliability of systems
 - References

- 2** Elicitation of subjective probability distributions
 - Definition
 - Copulas
 - Linear correlation
 - Dependence measures
 - References

Reliability approach in mechanics

- ▶ In mechanics, the term reliability describes the ability of a system to accomplish a required function
- ▶ Mechanical materials or structures are considered as systems comprising an input, a state and an output
- ▶ The classical point of view

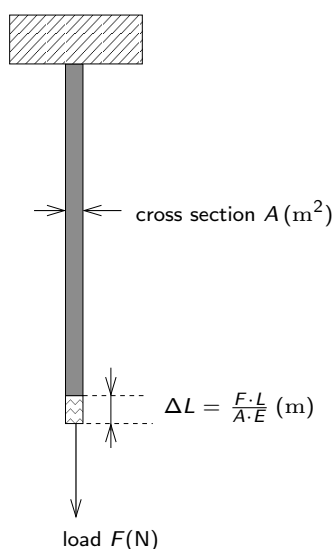


- ▶ The success of a design (or serviceability) is seen in the verification of the inequality

$$g_{R,S}(R, S) = R - S \geq 0$$

- ▶ The quantity $g_{R,S}(R, S) = R - S$ is called the (safety) margin
- ▶ $g_{R,S}$ is called the limit-state function.

Example: deformation of a rod



- ▶ The yield point of a material is defined as the stress at which a material begins to deform plastically (prior to the yield point the material will deform elastically and will return to its original shape when the applied stress is removed)
- ▶ Stress: $\sigma = F/A$ (Pa)
- ▶ Yield limit: σ_y (Pa)
- ▶ Design rule: $F/A \leq \sigma_y$
- ▶ Set

$$\begin{aligned} x &= (F, A, \sigma_y) \in \mathbb{R}^3 \\ r(x) &= A\sigma_y \\ s(x) &= F \end{aligned}$$

- ▶ Define $g_{R,S} : (R, S) \mapsto R - S$. The design must verify

$$g_{R,S}(r(x), s(x)) = A\sigma_y - F \geq 0$$

Failure region, Limit-state function

- ▶ The operating scenario is the availability of a resistance R greater than the stress S , such that

$$g_{R,S}(R, S) = R - S \geq 0$$

- ▶ The failure scenario is

$$g_{R,S}(R, S) = R - S < 0$$

and the failure region is defined as the region of \mathbb{R}_+^2

$$\Gamma = \{(r, s) \in \mathbb{R}_+^2; g_{R,S}(r, s) < 0\}$$

- ▶ The limit state is defined as the region of \mathbb{R}_+^2

$$\partial\Gamma = \{(r, s) \in \mathbb{R}_+^2; g_{R,S}(r, s) = 0\}$$

(hence, the name limit-state function for $g_{R,S}$)

Probability of failure

- ▶ In practice, the load applied on a system is unknown, and the design parameters are subjected to dispersions

→ the parameter vector x is uncertain, and can be modeled by a random vector

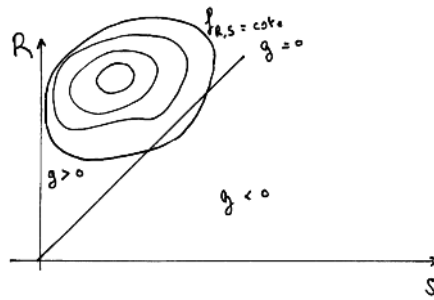
$$X \sim P_{\mathbb{X}}$$

- ▶ Then, $R = r(X)$ and $S = s(X)$ are also random variables
- ▶ Let $f_{R,S}$ be the joint pdf of (R, S) (wrt to the Lebesgue measure)
- ▶ The probability of failure of the system corresponds to

$$\alpha = P\{R - S < 0\} = \int_{r-s < 0} f_{R,S}(r, s) dr ds$$

- ▶ Reliability is defined as $1 - \alpha$

Probability of failure



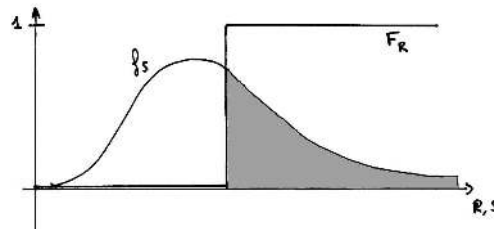
- Usually, R and S assumed to be independent
- Then,

$$\begin{aligned}\alpha &= \int_{r-s < 0} f_S(s) f_R(r) dr ds \\ &= \int_{s \in [0, \infty[} f_S(s) \left(\int_{r \in [0, s]} f_R(r) dr \right) ds \\ &= \int_{s \in [0, \infty[} f_S(s) F_R(s) ds\end{aligned}$$

where f_S and f_R are the pdf of S and R , and F_R is the cdf of R

Probability of failure: case of a known resistance

- When the resistance is deterministic: $R = r_0 \implies F_R(r) = \mathbb{1}_{r_0, \infty[}(r)$



- Then $\alpha = P\{S > r_0\} = P\{s(X) > r_0\}$

Reliability indexes in structural reliability

- ▶ In the domain of structural reliability, the serviceability of a design is often quantified using the notion of a **reliability index**
- ▶ When the resistance and the stress are deterministic, the notion of reliability index may be defined arbitrarily as the numbers

$$\frac{R}{S} \text{ or } R - S$$

- ▶ When R and S are viewed as independent random variables, a notion reliability index may be defined as the number

$$\beta_C = \frac{E(g_{R,S}(R, S))}{\text{var}(g_{R,S}(R, S))^{1/2}} = \frac{m_R - m_S}{\sqrt{\sigma_R^2 + \sigma_S^2}}$$

- ▶ However, the interpretation of β_C is not simple and, above all, β_C is generally not related to the probability of failure of the system

Probability of failure expressed in terms of the uncertain factors

- ▶ Note that the point of view of structural reliability is not different from that presented in Part I
- ▶ In practice, f_R and f_S cannot be determined directly by the user → only the distribution $P_{\mathbb{X}}$ of the vector of uncertain factors can
- ▶ Thus, it is generally easier to express a limit-state function $g_{\mathbb{X}}$ in the factor space:

$$g_{\mathbb{X}} : \mathbb{X} \mapsto g_{R,S}(r(x), s(x))$$

so that the probability of failure can be expressed as

$$\alpha = P\{g_{\mathbb{X}}(X) < 0\}, \quad X \sim P_{\mathbb{X}}$$

or

$$\alpha = P_{\mathbb{X}}\{x \in \mathbb{X}; g_{\mathbb{X}}(x) < 0\} = P_{\mathbb{X}}\{g_{\mathbb{X}} < 0\}$$

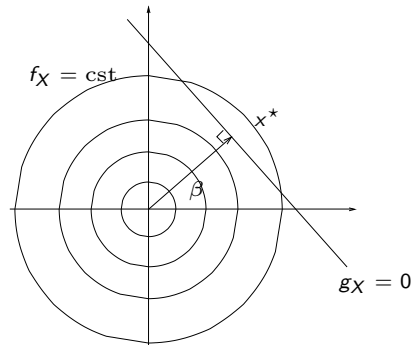
(The probability of failure is the volume of excursion of g above zero.)

Probability of failure for Gaussian random factors and affine limit-state function

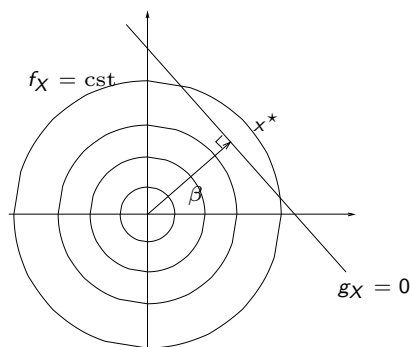
- ▶ Assume $X \sim N(0, \mathbb{I}_d) \in \mathbb{R}^d$
- ▶ Assume moreover that g_X is affine: $\forall x \in \mathbb{R}^d$

$$g_X(x) = a_0 + a_1 x_{[1]} + \dots + a_d x_{[d]} = a_0 + (a, x)$$

- ▶ Then, the limit-state $\partial\Gamma$ is the hyperplane defined by $a_0 + (a, x) = 0$



Probability of failure for Gaussian random factors and affine limit-state function



- ▶ Let x^* be the nearest point of the hyperplane $\partial\Gamma = \{x \in \mathbb{R}^d; a_0 + (a, x) = 0\}$ to the origin
- ▶ $x \in \partial\Gamma \iff (x - x^*, x^*) = (x^*, x) - \|x^*\|^2 = 0$
- ▶ Thus, for $a_0 \neq 0$,

$$\frac{a}{a_0} = -\frac{x^*}{\|x^*\|^2} \implies \beta := \|x^*\| = \frac{|a_0|}{\|a\|}$$

- ▶ Note that $U = (X, \eta)$, with $X \sim N(0, \mathbb{I}_d)$ and $\eta = x^*/\beta$, is a random variable with distribution $N(0, 1)$

Probability of failure for Gaussian random factors and affine limit-state function

- ▶ A failure corresponds to the event $\{U > \beta\} = \{(X, x^*) > \beta^2\}$, which has probability

$$\alpha = 1 - \mathbb{P}\{U \leq \beta\} = 1 - \Phi(\beta) = \Phi(-\beta)$$

- ▶ Therefore, to compute a probability of failure in the case of standard normal factors and affine limit-state function g_X :

1. Solve

$$x^* = \arg \min_{x \in \mathbb{R}^d} \|x\|$$

subject to $g_X(x) = 0$

2. The probability of failure is $\alpha = \Phi(-\beta)$, with $\beta = \|x^*\|$

- ▶ In the literature of structural reliability, x^* is called a **design point**, or **most central failure point**

Probability of failure for Gaussian random factors and affine limit-state function

- ▶ Consider again an affine limit-state function $g_X : x \mapsto a_0 + (a, x)$
- ▶ Define the *margin* as the random variable $Z = g_X(X)$
- ▶ Note that

$$E(Z) = a_0$$

and

$$\text{var}(Z) = a_1^2 + \dots + a_d^2 = \|a\|^2$$

- ▶ Thus,

$$\beta = \frac{|a_0|}{\|a\|} = \frac{|E(Z)|}{\text{var}(Z)^{1/2}}$$

- ▶ In the literature of structural reliability, the ratio $\frac{E(Z)}{\text{var}(Z)^{1/2}}$ is interpreted as a reliability index
- ▶ β is called the **Hasofer-Lind reliability index**

Measure of the importance of the factors with respect to a failure

- ▶ The sensitivity of the probability of failure with respect to changes in the factors is an important information for the design of a system (makes it possible to understand which factor are most important to control)
- ▶ One possibility to define a notion of sensitivity is to look at the variation of the margin $z = g_X(x)$ as a function of x ;
- ▶ We have

$$\nabla g_X(x) = a = -\frac{a_0}{\beta} \eta$$

with $\eta = \frac{1}{\beta} x^*$

- ▶ Thus, if the i^{th} component $\eta_{[i]}$ of the unit vector η is large, the margin will vary rapidly as we move along the i^{th} direction
- ⇒ $\eta_{[i]}$ accounts for the importance of the i^{th} factor

Extensions

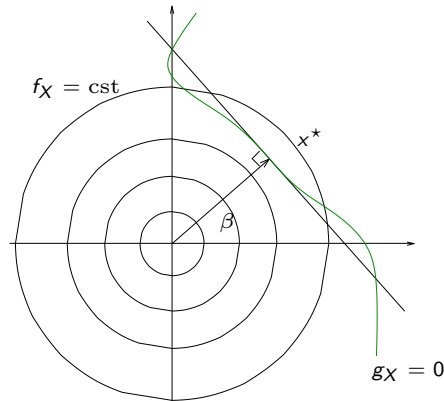
At this point, several extensions can be considered:

- ▶ Non-affine limit-state functions
- ▶ Gaussian non-standard random factors
- ▶ Non-Gaussian independent random factors
- ▶ Non-Gaussian non-independent factors

→ first-order reliability method (and related methods)

Non-affine limit-state functions

- ▶ In some applications, it may be reasonable to approximate the limit state $\partial\Gamma$ by a geometric shape such as a hyperplane



- ▶ Therefore, to approximate a probability of failure in the case of standard normal factors and a non-affine limit-state function g_X :

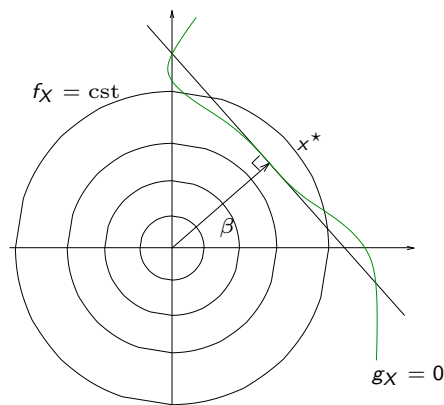
1. Solve

$$x^* = \arg \min_{x \in \mathbb{R}^d} \|x\|$$

subject to $g_X(x) = 0$

2. An approximation of the probability of failure is $\hat{\alpha} = \Phi(-\beta)$, with $\beta = \|x^*\|$

Non-affine limit-state functions: FORM



- ▶ If g_X is smooth, note that $\nabla g_X|_{x=x^*} \propto \eta$, with $\eta = x^*/\beta$
- ▶ Thus the equation of the approximating hyperplane is

$$(\nabla g_X|_{x=x^*}, x - x^*) = 0$$
- ▶ Note that $x \mapsto (\nabla g_X|_{x=x^*}, x - x^*)$ is the first-order approximation of g_X in the neighborhood of x^*
- ▶ In structural reliability, using a first-order approximation of g_X at x^* in a standard Gaussian factor space is called **First-Order Reliability Method (FORM)**

Finding the most central failure point

► Solving

$$x^* = \arg \min_{x \in \mathbb{R}^d} \|x\|$$

subject to $g_X(x) = 0$

is a constrained optimization problem, for which many algorithms has been proposed in the literature:

- Penalty methods
- Augmented Lagrangian
- Projected gradient
- BFGS
- SQP
- ...

(they all are local optimization algorithms)

- In general, finding a good approximation of x^* can be done with a moderate number of evaluations of g_X
- However: g_X generally need to be convex and differentiable
 - If there exist (even small) numerical instabilities when computing g_X , as may be the case in models based on the numerical solution of some partial differential equations, then the optimization of g_X using standard techniques can fail direly

How much can go wrong with FORM?

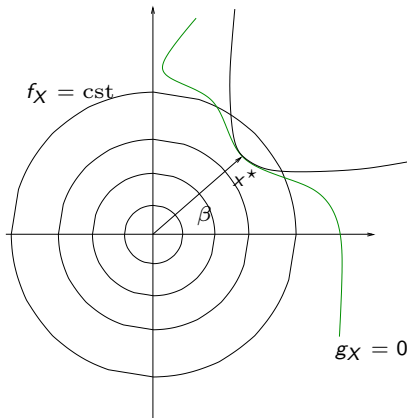
- For some applications, finding x^* can be done with only a few evaluations of $g_X \rightarrow$ interesting when g_X is expensive to evaluate
- However, having found x^* does not tell if the approximation $\alpha \approx \Phi(-\beta)$ is good or not
- The probability of failure can be overestimated or underestimated
- Consider, for instance the following domain of failure:

$$\Gamma = \{x \in \mathbb{R}^d; \|x\| > \beta_0\}$$

- Let $V = X_{[1]}^2 + \dots + X_{[d]}^2$, so that $V \sim \chi^2(d)$. The failure event is $\{V > \beta_0^2\}$.
- Hence, we have $\alpha = 1 - F_V(\beta_0^2)$ and $\hat{\alpha} = 1 - \Phi(\beta_0)$
- Example: suppose $d = 20$ and $\beta_0 = 5$, we obtain $\hat{\alpha} \approx 2.9 \cdot 10^{-7}$ but $\alpha \approx 0.2!$
- The FORM approximation should be used only when prior knowledge about the shape of $\partial\Gamma$ is available

Quadratic approximation: SORM

- ▶ Instead of using a first-order approximation, one could think of using a second-order approximation



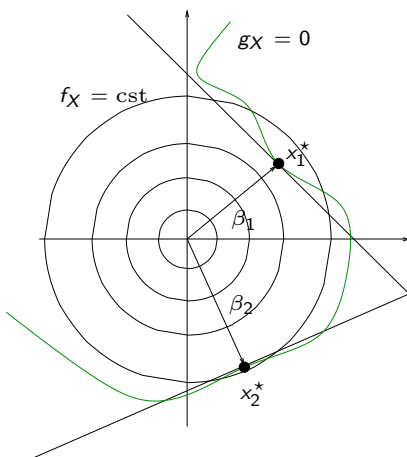
→ Second-Order Reliability Method (SORM)

- ▶ Such an approximation requires the approximation of x^* and the estimation of the curvature of the limit-state at x^*
- ▶ A quadratic approximation is more expensive to obtain, but there is, in general, **no guarantee that the approximation of the probability of failure will be better than the first-order approximation**
- ▶ In fact, we can have

$$\alpha > \hat{\alpha}_{\text{FORM}} > \hat{\alpha}_{\text{SORM}}$$

Multiple local first-order approximations: multi-FORM

- ▶ Another possibility proposed in the literature: compute several design points, that is, several local minimizers of $\|x\|$ subject to the constraint $g_X(x) = 0$



- ▶ Finding several local minimizers can be a difficult task
- ▶ Consider again the domain of failure:

$$\Gamma = \{x \in \mathbb{R}^d; \|x\| > \beta_0\}$$

and the approximation

$$\hat{\Gamma} = \{x \in \mathbb{R}^d; \max_i |x_{[i]}| > \beta_0\}$$

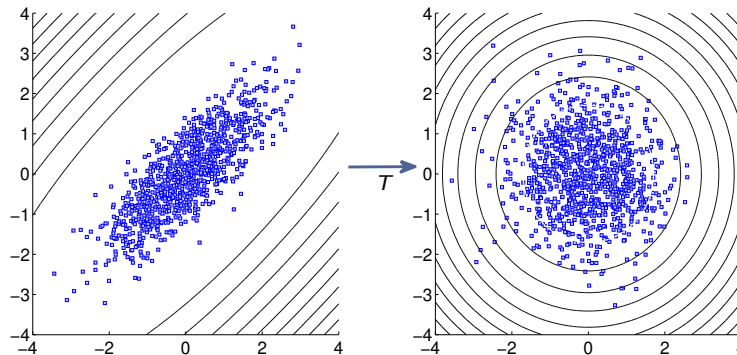
(obtained with $2d$ design points)

- ▶ Then,
 $\hat{\alpha} = P\{X \in \hat{\Gamma}\} = 1 - \prod_i (\Phi(\beta_0) - \Phi(-\beta_0)) = 1 - (1 - 2\Phi(-\beta_0))^d \approx 2d\Phi(-\beta_0)$

⇒ for $d \gg 1$, $\alpha \gg \hat{\alpha}$

Non-standard Gaussian random factors

- ▶ Assume $X \sim N(m, K) \in \mathbb{R}^d$, with $m \in \mathbb{R}^d$ and K a $d \times d$ symmetric definite positive (SDP) matrix
- ▶ To apply the framework above, the idea is to search for a one-to-one whitening transformation $T : \mathbb{R}^d \rightarrow \mathbb{R}^d$ such that $U = T(X)$ is a standard Gaussian random vector
- ▶ Consider the eigendecomposition of K such that $K = Q\Lambda Q^T$, where Λ is the diagonal matrix of the eigenvalues of K , and Q is orthogonal
- ▶ Then, $T : X \mapsto \Lambda^{-1/2}Q^T(X - m)$ is such that $U = T(X) \sim N(0, \mathbb{I}_d)$
- ▶ Note that if g_X is affine, then $g_U = g_X \circ T^{-1}$ is also affine in the standardized space



Non-Gaussian independent random factors

- ▶ When the components of X are independent but non Gaussian, the idea is to search for one-to-one transformations $T_i : \mathbb{R}^d \rightarrow \mathbb{R}^d$ such that for each $i = 1, \dots, d$, $U_{[i]} = T_i(X_{[i]})$ is a standard Gaussian random vector
- ▶ Assume that the cdf F_i of $X_{[i]}$ is continuous and strictly increasing, then

$$U_{[i]} = \Phi^{-1}(F_i(X_{[i]})) \sim N(0, 1),$$

where Φ^{-1} stands for the inverse (reciprocal) function of Φ
Indeed,

$$\begin{aligned} P\{U_{[i]} \leq u\} &= P\{\Phi^{-1}(F_i(X_{[i]})) \leq u\} \\ &= P\{X_{[i]} \leq F_i^{-1}(\Phi(u))\} \\ &= \Phi(u) \end{aligned}$$

- ▶ Thus, if the components of X are independent but non Gaussian, the random vector

$$U = T(X) = (\Phi^{-1} \circ F_1(X_{[1]}), \dots, \Phi^{-1} \circ F_d(X_{[d]})) \sim N(0, \mathbb{I}_d)$$

Non-Gaussian non-independent random factors

- ▶ As above, the idea is to transform the random vector of factors
- ▶ This case will be examined separately: in fact it raises **two difficult issues**:
 - ▶ how to specify the distribution of a random vector in the case of non-independent components?
 - ▶ what is the influence of the input distribution on the probability of failure?

FORM and related methods: summing up

- ▶ Very popular methods in the domain of structural reliability
- ▶ FORM is easy to understand and to implement
- ▶ When a first-order approximation is relevant, a good approximation of the probability of failure can be found with a small number of function evaluations
- ▶ A major pitfall: the approximation of the probability of failure can be significantly wrong

A personal perspective on the geometrical approximation approaches

- ▶ If g_X is not expensive to evaluate:
 - ▶ Monte Carlo should be preferred over any geometrical approximation
- ▶ If g_X is expensive:
 - ▶ a simple MC approach cannot be used
 - ▶ what can be the use of a geometrical approximation?
- ▶ It can be very wrong to approximate the limit state with a geometrical shape
 - ▶ can only be justified when it is known in advance that a given geometrical approximation is correct
- ▶ SORM can be thought as a correction of FORM, but from a mathematical perspective, it is not \rightarrow using SORM over FORM can only be justified when it is known in advance that g_X is almost quadratic
- ▶ A multi-FORM approach seems preferable, but using it is to admit that the shape of the limit state is unknown, which is dangerous for a geometrical approximation approach
- ▶ Fortunately, in a large number of applications, the limit-state function is almost affine, which explains why FORM remains a very popular method

Reliability of systems

- ▶ Until now, we have implicitly considered the case of the failure of a unique component
- ▶ In a real system, a failure can happen due to the failure of just one of its (possibly many) components
- ▶ The designer can also choose to have redundancy on critical components; in this case the failure of the system happens when all redundant components fail
- ▶ To deal with these issues, the domain of structural reliability generally introduces the notions of **parallel** and **series** systems

Reliability of systems: series systems

- ▶ Let x denotes the state of a system, and let Γ be a domain of failure
- ▶ In structural reliability, a system is called a *series system* if the occurrence of one single failure event brings a failure on the whole system
- ▶ The classical example is that of a chain whose failure is related to any of its links
- ▶ In other words, it means that we can write

$$\Gamma = \bigcup_{i=1}^l \Gamma^{(i)}$$

- ▶ Assume that each domain of failure $\Gamma^{(i)}$ is characterized by a limit-state function $g_X^{(i)}$, so that the failure of the i^{th} component corresponds to the event $\{g_X^{(i)}(X) < 0\}$
- ▶ Then the failure event for the whole system can be characterized by the limit-state function

$$g_X : x \mapsto g_X^{(1)}(x) \wedge \cdots \wedge g_X^{(l)}(x)$$

- ▶ Conclusion: the case of series systems can be dealt with using the framework we have exposed previously

Reliability of systems: parallel systems

- ▶ In structural reliability, a system is called a *parallel system* if the failure of all events is necessary for the failure of the whole system
- ▶ A parallel system is a principle a redundancy
- ▶ Using the notations above, the domain of failure for a parallel system corresponds to

$$\Gamma = \bigcap_i \Gamma_i$$

- ▶ Again, the failure event for the whole system can be characterized by a single limit-state function

$$g_X : x \mapsto g_X^{(1)}(x) \vee \cdots \vee g_X^{(l)}(x)$$

- ▶ Conclusion: parallel systems can also be dealt with using the framework we have exposed previously

Some references

- ▶ Ditlevsen O. and Madsen H.O. (1996), Structural reliability methods, Wiley
- ▶ Lemaire M., Chateauneuf A., and Mitteau J.C., (2009) Structural Reliability, Wiley

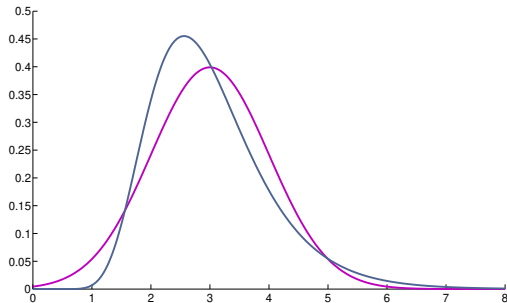
Elicitation of subjective probability distributions

- ▶ A very difficult issue: to choose a probability distribution for modeling the uncertain factors of a system
- ▶ The process of choosing a distribution for the factors is called elicitation¹ in the literature of decision analysis
- ▶ Elicitation is particularly difficult when doing risk analysis about new and untried technologies, for which little data are available
- ▶ Very often, risk analysis relies on expert judgment
- ▶ Elicitation of subjective probability distributions is often subject to a number of serious biases, such as overconfidence in the ability to quantify uncertainty

¹from Latin elicere: draw forth, bring out

Elicitation of subjective probability distributions

- ▶ Assume that we are given an approximation of the mean and the standard deviation of a random variable
- ▶ An experiment:



- ▶ $X \sim N(\mu, \sigma^2)$
- ▶ $Y \sim \text{logN}(m, s^2)$, with $s^2 = \log\left(\frac{\sigma^2}{\mu^2} + 1\right)$ and $m = \log \mu - s^2/2$.
- ▶ Then, $E[X] = E[Y] = \mu$ and $\text{var}[X] = \text{var}[Y] = \sigma^2$.
- ▶ Assume $\mu = 3$ and $\sigma^2 = 1$. Then, $P(X > 7) = 3.1 \cdot 10^{-5}$ and $P(Y > 7) = 2.8 \cdot 10^{-3}$

- ▶ For risk analysis, “knowing” only the mean and the standard deviation of a random variable is a very poor information
- ▶ In fact, in many applications, the occurrence of a failure is likely to be related to an extreme event in the factor space
→ it is probably more important to characterize the **tail behavior** of the factors than the central behavior (EVT can help)

Elicitation of subjective probability distributions

- ▶ Elicitation gets even more complicated if we want to introduce dependence information between random variables. In fact, the question is: “how to measure dependence?”
- ▶ In probability theory and statistics, the dependence between random variables is described by the concept of copula (from Latin co-apere “join together”)

Copulas

- ▶ Consider a random vector $X = (X_1, \dots, X_d) \in \mathbb{R}^d$. The dependence between the component random variables X_1, \dots, X_d is completely described by the joint cdf

$$F(x_1, \dots, x_d) = P\{X_1 \leq x_1, \dots, X_d \leq x_d\}$$

- ▶ For simplicity, assume that the components X_i , $i = 1, \dots, d$, have continuous, strictly increasing, marginal cdfs F_i
- ▶ The concept of copula: separate F into a part that describes the **dependence structure** and parts which describe the **marginal behavior** only
- ▶ Transform X component-wise to obtain standard-uniform marginal distributions $U([0, 1])$

$$T: \begin{array}{l} \text{dom } F \quad \rightarrow \quad [0, 1]^d \\ (x_1, \dots, x_d) \mapsto (F_1(x_1), \dots, F_d(x_d)) \end{array}$$

- ▶ The joint cdf of $U = T(X)$ is called the **copula** of the random vector X
- ▶ It follows that for $x \in \text{dom } F$

$$F(x_1, \dots, x_d) = C(F_1(x_1), \dots, F_d(x_d))$$

and for $u \in [0, 1]^d$

$$C(u_1, \dots, u_d) = F(F_1^{-1}(u_1), \dots, F_d^{-1}(u_d))$$

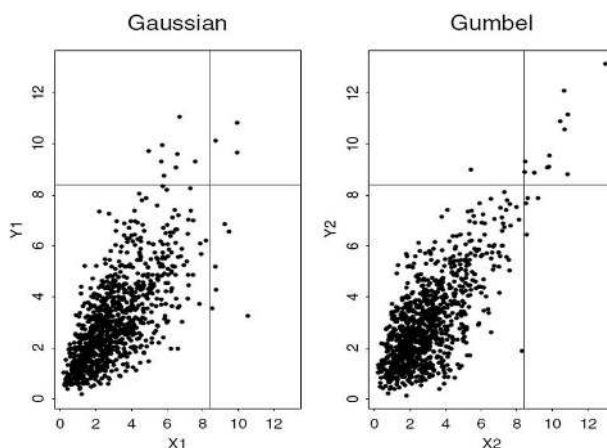


Copulas

- ▶ Some copulas/dependence structures:
 - ▶ The independent copula: $C_{\text{ind}}(u) = u_1 u_2 \cdots u_d$
 - ▶ The Gaussian copula

$$C_{g,R}(u_1, \dots, u_d) = \int_{-\infty}^{\Phi^{-1}(u_1)} \cdots \int_{-\infty}^{\Phi^{-1}(u_d)} (2\pi)^{-d/2} (\det R)^{-1/2} \exp\left(-\frac{1}{2} u^T R^{-1} u\right) du_1 \cdots du_d$$

- ▶ The bivariate Gumbel copula $C_{G_{u,\beta}}(u, v) = \exp\left[-\{(-\log u)^{1/\beta} + (-\log v)^{1/\beta}\}^\beta\right]$



from Embrechts et al. (1999)

Realizations from two distributions with identical Gamma(3, 1) marginal distributions and identical correlation $\rho = 0.7$, but different dependence structures.



Linear correlation (or Pearson's correlation)

- ▶ In some applications, choosing independent random factors is irrelevant
- ▶ However, choosing a dependence structure can be a difficult task because information may be scarce (e.g., no data available)
- ▶ Very often, the dependence structure of a random vector is summarized through (linear) correlations between components

Recall that for $(X, Y) \in \mathbb{R}^2$, a second-order random vector, the linear correlation coefficient is

$$\rho(X, Y) = \frac{\text{cov}(X, Y)}{\sqrt{\text{var}(X) \text{var}(Y)}}$$

If $|\rho(X, Y)| = 1$, then there is a perfect linear dependence between X and Y , i.e., $Y = aX + b$, for some $a, b \in \mathbb{R}$

Nataf transformation

- ▶ The **Nataf transformation** T_{Nataf} is a one-to-one function which maps a random vector X with a Gaussian copula to a random vector U with standard Gaussian distribution
- ▶ Makes it possible to apply FORM for non-Gaussian non-independent random vectors
- ▶ Conversely, the Nataf transformation makes it possible to define implicitly the distribution F of a random vector $X = (X_1, \dots, X_d)$ with
 1. Gaussian copula
 2. prescribed continuous, strictly increasing, marginals F_1, \dots, F_d
 3. prescribed correlation matrix $R_X = (\rho(X_i, X_j))_{i,j}$
- ▶ Considering a Gaussian copula for the dependence structure of X should not be considered as a canonical choice \rightarrow a comprehensive risk analysis procedure should assess the consequences of this particular choice

Inverse Nataf transformation

- ▶ Let $U \sim N(0, I_d)$
- ▶ Given continuous strictly increasing marginals cdfs F_1, \dots, F_d , and a $d \times d$ correlation matrix R_X , the inverse Nataf transformation is defined as $X = T_{\text{Nataf}}^{-1}(U) = T_3 \circ T_2 \circ T_1(U)$, with

$$T_1: \begin{array}{l} \mathbb{R}^d \\ u = (u_1, \dots, u_d) \end{array} \begin{array}{l} \rightarrow \\ \mapsto \end{array} \begin{array}{l} \mathbb{R}^d \\ Cu \end{array}$$

$$T_2: \begin{array}{l} \mathbb{R}^d \\ v = (v_1, \dots, v_d) \end{array} \begin{array}{l} \rightarrow \\ \mapsto \end{array} \begin{array}{l} [0, 1]^d \\ (\Phi(v_1), \dots, \Phi(v_d)) \end{array}$$

$$T_3: \begin{array}{l} [0, 1]^d \\ w = (w_1, \dots, w_d) \end{array} \begin{array}{l} \rightarrow \\ \mapsto \end{array} \begin{array}{l} \mathbb{R}^d \\ (F_1^{-1}(w_1), \dots, F_d^{-1}(w_d)) \end{array}$$

where C is a $d \times d$ matrix which is computed in such a way that X has correlation matrix R_X

- ▶ Note that $V = T_1(U)$ is a Gaussian vector. Thus, it has a Gaussian copula. Since T_2 and T_3 are component-wise monotonic transformations, T_2 and T_3 are copula-invariant
- ▶ Note that the correlation matrix of $V = T_1(U)$ is $R_V = CC^T$. In general, $R_V \neq R_X$.
- ▶ Note also that it is not always possible to prescribe any correlation coefficient (depending on the choice of the marginals)

Dependence measures

- ▶ As mentioned above, considering a Gaussian copula for the dependence structure of X should not be considered as a canonical choice
- ▶ Moreover, measuring dependence based on correlation coefficients can be misleading
→ linear dependence should not be taken as a canonical dependence measure

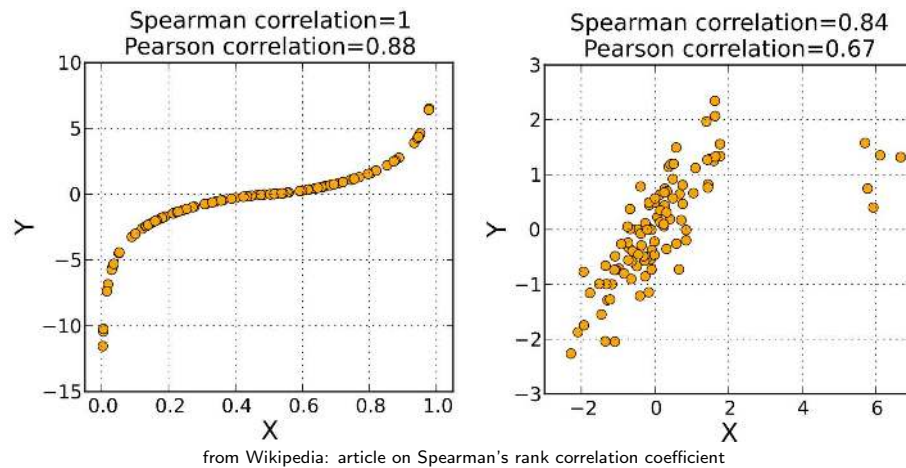


Realizations from seven bi-variate distributions with zero correlation

- ▶ Other measures of the dependence?

Dependence measures

- ▶ Non-linear dependence measures have been proposed in the literature
- ▶ In particular, rank correlation coefficients, such as Spearman's rank correlation coefficient ρ , and Kendall's rank correlation coefficient τ measure the extent to which two variables increase or decrease simultaneously
- ▶ For instance, Spearman's ρ is defined as the Pearson's correlation coefficient between the ranked variables



Tail dependence

- ▶ In the context of risk analysis, it might be relevant to study the dependence between extreme values
- ▶ Let $X = (X_1, X_2)$ be a vector of continuous random variables with marginals F_1 and F_2 . The coefficient of upper tail dependence of (X_1, X_2) is

$$\lambda_U = \lim_{u \rightarrow 1} P\{X_2 > F_2^{-1}(u) \mid X_1 > F_1^{-1}(u)\}$$

provided that the limit exists. If λ_U is well-defined, $\lambda_U \in [0, 1]$

- ▶ The value of λ_U is a property of the copula of X only
- ▶ Examples:
 - ▶ Consider the Gaussian bi-variate copula, with correlation $\rho < 1$. Then, $\lambda_U = 0$; that is, a Gaussian copula with $\rho < 1$ does not have tail dependence
 - ▶ The Gumbel bi-variate copula, with parameter $\beta > 1$, has tail dependence $\lambda_U = 2 - 2^{1/\beta}$
- ▶ Choosing a copula with tail-dependence over the Gaussian copula can modify the probability of failure by several orders of magnitude—see, for instance, Dutfoy and Lebrun, Congrès Français de Mécanique (2007)

Some references

- ▶ Embrechts P., McNeil A., Straumann D. (1999), Correlation and dependence in risk management: properties and pitfalls
- ▶ Embrechts P., Lindskog F., McNeil A. (2001), Modelling Dependence with Copulas and Applications to Risk Management
- ▶ Lebrun R. and Dutfoy A. (2009), A generalization of the Nataf transformation to distributions with elliptical copula